

Seismic Analysis of Shear Wall and Bracing in all Seismic Zones at Different Location on Multi-storey RCC Building

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ABSTRACT- Multi Storyed Building developments have been rapidly increasing worldwide. The growth of multistory building in the previous decades is observed as the part of necessity for vertical expansion for business and residence in urban areas. It is seen that there is requirement to study the structural systems for R.C.C framed structure, which endures the lateral loads due to seismic effect. Safety and minimum damage level of a structure could be the most essential need of tall buildings. To reach these needs, the structure must be of adequate lateral strength, lateral stiffness and sufficient ductility. From the different structural systems, shear wall frame or braced concrete frame could be a priority for designers. Hence, it draws in to audit and notice the conduct of these underlying frameworks under seismic impact. This research work centers on the correlation of seismic investigation of private structures utilizing supporting and shear walls. The investigation of the building is conveyed in each of the four seismic which are Zone II, Zone III, Zone IV and Zone V. This investigation contains understanding the key parts responsible for the construction to perform severely during a seismic tremor, with the goal that they acquire their reasonable attributes for the further quakes. Demonstrating of the design will be done through STAAD professionals. V8i programming.

Keyword- Shear walls, Bracing, STAAD professional V8i software, axial force, bending moment.

I. INTRODUCTION

The growth in recent multi- storyed building construction are the ones which started in the late nineteenth century, is intended largely for commercial and residential purposes. The establishment of tall buildings mainly comprises of an imaginary design, estimated study, preparatory design and optimization, to cautiously carry gravity as well as lateral loads. The principles of design are, strength, serviceability, stability and human comfort. Earthquakes nowadays occurs frequently all over the world. It is extremely hard to foresee its characteristics like intensity, location, and time of occurrence of earthquake. Structures suitably designed for usual loads like dead, live, wind etc. may not be necessarily safe against earthquake loading. The design of structures to stay within the elastic limit during earthquakes is neither feasible nor economically viable. The design approach is as per the Indian Code IS 1893(Part I): 2002 'Criteria for Earthquake Resistant Design of Structures' is to ensure that structures possess at least a minimum strength to endure slight earthquake happening frequently, without damage; bear moderate earthquakes with no substantial structural damage, but there could be some non-structural damage; and aims that structures withstand major earthquake without collapse. In order to safely withstand broad lateral forces

that are placed on them during regular earthquakes, structures need to have sufficient earthquake resistance features. Common structures for houses are mostly constructed to carefully carry their own loads. Low wind-induced lateral loads, gives poor performance under significant lateral forces triggered by even mild earthquakes.

A. Bracing

The use of a steel setting structure is a possible decision for retrofitting an upheld generous edge for dealt with seismic shows. Steel upholds give required strength and robustness, consume less room, are easy to manage during improvement, can similarly be used as an underlying part and is monetary. Steel upholds are convincing as they take up center point stresses and on account of their solidness, decline evasion alongside the heading of their bearing.

B. Shear wall

Shear divider is an upward part that can oppose horizontal powers coordinated along its direction. Shear dividers are primary framework comprising of supported boards, otherwise called Shear Panels. Substantial Shear dividers are far reaching in numerous tremor inclined nations like Canada, Turkey, Romania, Colombia, and Russia.

C. Objectives of Work

The recent study is an attempt towards analysis of the structure during the earthquake.

1. To make a residential building is analyzed, RC outlined structure considering distinctive seismic tremor forces II, III, IV and V by reaction spectra technique and track down the base shear an incentive for various constructions.
2. To carry out the Seismic analysis of RC frame with bare and different position of shear wall and braced frame is carried out using Linear static analysis method as per IS 1893 (Part I): 2002[22] by using STAAD-PRO software.
3. To analysis various sorts of models are thought of and examination of seismic execution is completed.
4. To analyze the models for hub powers, minutes, sidelong removals, max shear power and max twist and graphical and even portrayal of the information is introduced.

II. LITERATURE REVIEW

The research of various authors has been portrayed further.

M. S. Speicher et al. (2019) Framework gives both reemerging and damping in an adaptable game plan. Driven by SMA's interesting capacity to recuperate strains of up to around 8% through dispersion less stage change, the foundation of the propping proposed thus is the capacity to change the energy scattering in a returning hysteretic circle using an AQ game plan. The framework kept up with strength, pliability, and reappearing subsequent to being cycled to 2% float, which is a commonplace most extreme in underlying frameworks if non-primary components are to be protected. An insightful contextual analysis exhibited that shape memory compound frameworks will in general circulate the deformity all the more equally over the tallness of the design contrasted with customary frameworks, which is an advantageous seismic presentation trademark. It is imagined that, by utilizing a similar fundamental supporting arrangement, a wide scope of power twisting reactions can be available to an architect.

Montuori R. et al. (2018) intended to research the impact of the supporting plan on the seismic exhibitions of Moment Resisting Frames-Eccentrically Braced Frames (MRF-EBF) double frameworks, planned by two plan draws near: the first is the Theory of Plastic Mechanism Control (TPMC) while the subsequent one depends on Euro code 8 (EC8) plan arrangements In examination with the upset Y-conspire, the ghastly speed increase prompting the breakdown decreases on normal of about 10%, 20% and 35% if there should be an occurrence of K-plot, D plan and V-conspire, individually. Specifically, if there should be an occurrence of constructions planned by TPMC the outcomes acquired show that V-plot structures consistently

display the most noticeably awful exhibitions autonomously of the quantity of stories.

III. METHODOLOGY

The present study is an exertion towards investigation of the design during the tremor. G+14 stories private structure is thought of. To dissect a multi-storeyed RC outlined structure considering distinctive quake forces II, III, IV and V Zone by reaction spectra technique and track down the base shear an incentive for various constructions. Seismic examination of RC outline with exposed and diverse situation of shear divider and supported casing is completed utilizing Linear static investigation strategy according to IS 1893 (Part I): 2002 by utilizing STAAD-PRO programming .For this investigation various kinds of models are thought of and correlation of seismic execution is done

The methodology worked out to achieve the mentioned objectives is as follows:

1. Modeling of the selected building in Staad pro. V8i Software.
2. Retrieved time period of structure from the software.
3. Nine models as per the Indian code specification were prepared with II to V Zone.
 - (a) Models including Bare frame
 - (b) Frames with shear walls
 - (c) Frames with bracings.
4. Applied calculated Lateral seismic forces and load combinations as per IS 1893-2002.

Analyzed the models for axial forces, moments, lateral displacements, max shear force and max torsion and graphical and tabular representation of the data is presented.

IV. METHODS OF ANALYSIS

A. Equivalent static analysis

All plans against tremor burden ought to be considered on the unique idea of the heap. Nonetheless, for conventional general designs, investigation by equal straight examination technique is adequate. This is permitted in many activities for ordinary, low-ascent structures. Dynamic examination is excluded from this framework, be that as it may, it is assessed to be answerable for the preparation of the venture. First and foremost, the plan base shear is determined for the whole structure, and afterward it is flowed with the tallness of the structure. At each floor level, in this way got, the sidelong powers are circulated for various side burden opposition components. (Duggal S.K., 2010).

B. Nonlinear Static Analysis-

This is an advantageous technique wherein the investigation is done under perpetual vertical burden and continuously builds the horizontal burden to appraise the example of twisting and harm to the design. Nonlinear static examination is the strategy for seismic assessment wherein the design is addressed by the direct twist, which exhibits

the association between the base shear force and the removing of the housetop. It is generally called sucker assessment.

C. Response Spectrum Method

In this strategy, top reactions of a construction are gotten straight by tremor reactions during quake. The most extreme response is made for the undamped ordinary period next and for different sprinkling regards, and can be conveyed the extent that most noteworthy relative speed or most outrageous relative removing. (Duggal S.K., 2010).

D. Seismic Analysis As Per IS: 1893-2002

The accurate seismic analysis of the structure is extremely complex and to deal with this complexity, the number of researches was done in a sophisticated and easy manner to design the earthquake resistant structures with the purpose of dealing with the complex dynamic effects of seismic induced force in the structures. Various methods of seismic analysis have been developed to determine lateral force, which are completely linear elastic to non-linear incompatible analysis.

Many of the analysis techniques are being used in design and incorporated in codes of practices of many countries. However, since in the present study our main focus is on the Indian Standard codal provisions, the method of analysis described in IS 1893 (Part 1): 2002 are presented in this paper.

E. Load Combinations

Load combinations that are to be used for Limit state Design of reinforced concrete structure are listed below.

1. 1.5(DL+LL)
2. 1.2(DL+LL±EQ-X)
3. 1.2(DL+LL±EQ-Y)
4. 1.5(DL±EQ-X)
5. 1.5(DL±EQ-Y)
6. 0.9DL±1.5EQ-X
7. 0.9DL±1.5EQ-Y

V. STRUCTURAL MODELLING

A. Modeling of Building Frame

Metallic braces is the easiest and shear walls simplest way of reducing response of building which gave rise to nine models for the analysis

1. Model in -BFB- Bare frame RCC Building
2. Model in -BX1- Framed building with Bracing at the exterior side along X-direction.
3. Model in -BY2- Framed building with Bracing at the exterior side along Z-direction.
4. Model in -BXY3- Framed building with Bracing at the exterior side along X and Z-direction.
5. Model in -BEC4- Framed building with Bracing at the exterior side around the corners.
6. Model in -SW1- Framed building with Shear wall at the exterior side along X-direction.

7. Model in -SW2- Framed building with Shear wall at the exterior side along Z-direction.
8. Model in -SW3- Framed building with Shear wall at the exterior side along X and Z-direction.
9. Model in -SW4- Framed building with Shear wall at the exterior side around the corners.

This arrangement of supporting is utilized in light of the fact that offbeat propping frameworks comprise of a connection component that goes through inelastic twisting for energy dispersal

Table 1. Specifications of the building

Specifications	Data
Model	G+14
Plan Size	28m x 21m
Plan Size	588m ²
Floor to Floor Height	3m
Total Building Height	45
No. of bays along X direction	6
No. of bays along Z direction	8
Bay Length along X direction	3.5m
Bay Length along Z direction	3.5m
Concrete grade used	M 30
Frame type	SMRF
Column size	0.40m X 0.50m
Beam size	0.30m X 0.40m
Transverse Beams	0.25m X 0.35m
Slab Thickness	0.115m
Inner Wall Thickness	0.115m
Outer wall	0.23m
Density of Brick	20 kN/m ³
Grade of Concrete	M-30
Unit Weight of Concrete	25 kN/m ³
Grade of Steel	Fe 415
Seismic Zone	Zone II,III,IV,V
Zone Factor corresponding to seismic zone	0.10,0.16,0.24,0.36
Importance Factor	1.0
Live Load	3.5 kN/m ³
Floor finish	1 kN/m ³
Depth of Foundation	2.5 m
Soil Type	Medium Soil
Damping Ratio	5%
Size of thickness of shear wall	0.2 m
Section for steel bracing	ISA 110 X 110 X 10mm

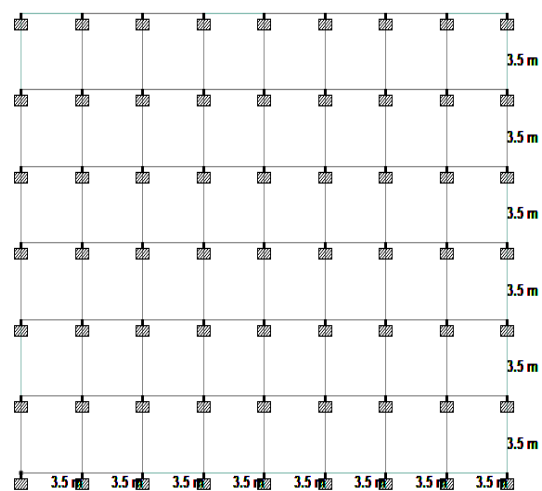


Figure 1. Plan of the building

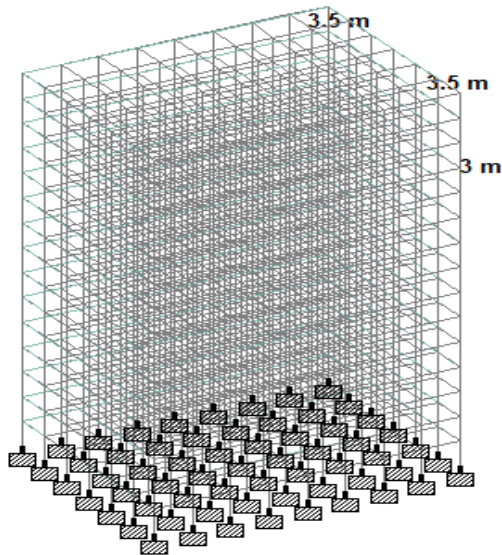


Figure 2. Model of the building

B. Braced Frame Modeling

For supports point area ISA 60 X 40 X 6 is utilized. There are four preliminary areas in the structure where supports are set and investigated for their impact on sidelong solidness. Supports are demonstrated as pivotal power individuals having stuck end associations. Bracings are of X-type demonstrated all through the stature of the structure. The four areas are as per the following:

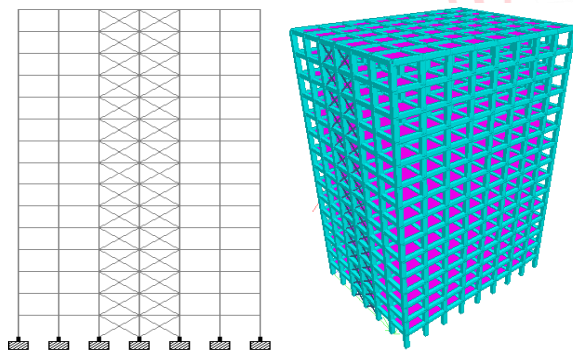


Figure 3. Framed building with Bracing at the exterior side along X-direction

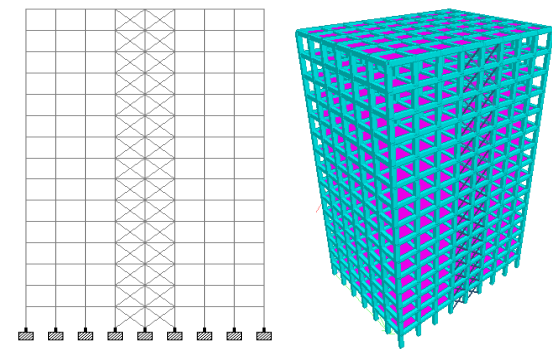


Figure 4. Framed building with Bracing at the exterior side along z-direction.

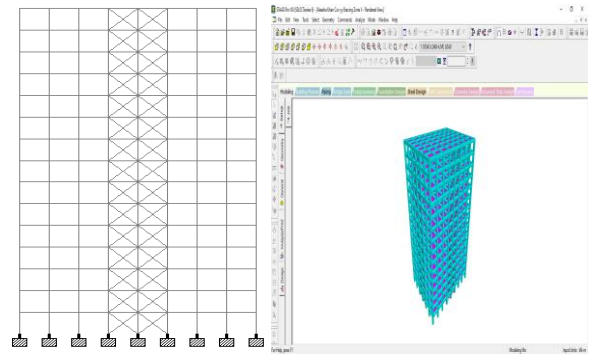


Figure 5. Framed building with Bracing at the exterior side along X and z-direction.

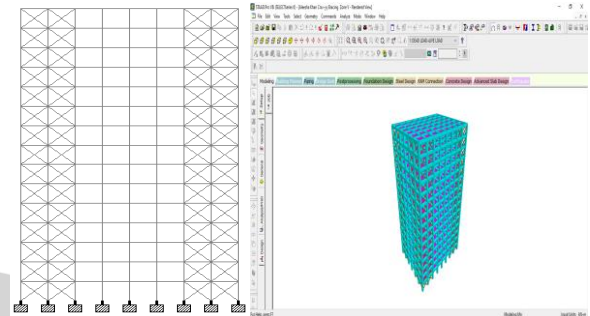


Figure 6. Framed building with Bracing at the exterior side around the corners.

C. Modeling of Shear Wall Frame

Shear Wall considered is of 250mm thickness, and put along the whole stature of the construction. The shear walls are placed in the exact locations as that of bracings, and the analysis is done. The four locations are as follows:

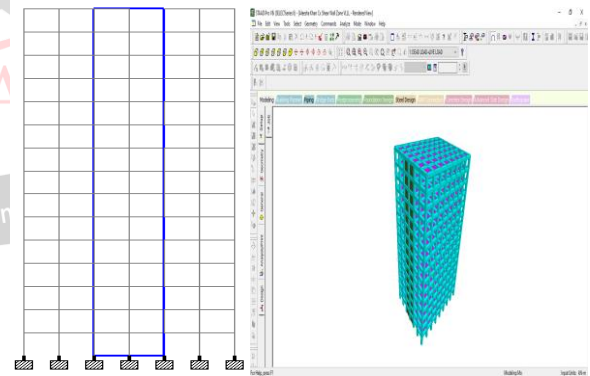


Figure 7. Framed building with Shear wall at the exterior side along X-direction.

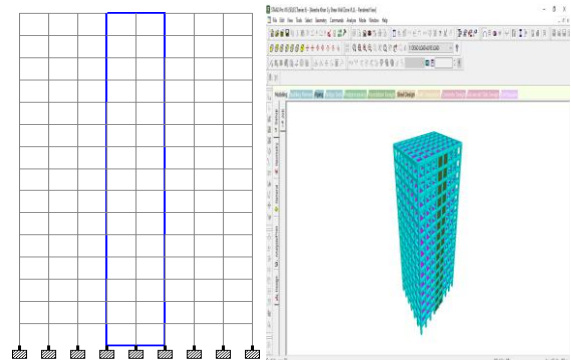


Figure 8. Framed building with Shear wall at the exterior side along Z-direction.

Zone	Soil Type	Model Type	Max. Axial Force KN-m
II	Medium	BFB	3084.187
		BX1	3105.122
		BY2	3060.097
		BXY3	3237.967
		BEC4	3368.47
		SW1	3446.125
		SW2	3009.234
		SW3	2703.727
III	Medium	BFB	3662.431
		BX1	3658.706
		BY2	4947.643
		BXY3	3660.097
		BEC4	3965.621
		SW1	5141.555
		SW2	3446.125
		SW3	3009.234
IV	Medium	BFB	4465.085
		BX1	4131.810
		BY2	5976.484
		BXY3	4028.348
		BEC4	4787.607
		SW1	6211.149
		SW2	3453.888
		SW3	3016.997
V	Medium	BFB	5388.176
		BX1	5684.934
		BY2	5532.102
		BXY3	6744.801
		BEC4	5300.399
		SW1	5884.934
		SW2	5673.765
		SW3	5561.863
		SW4	5530.950

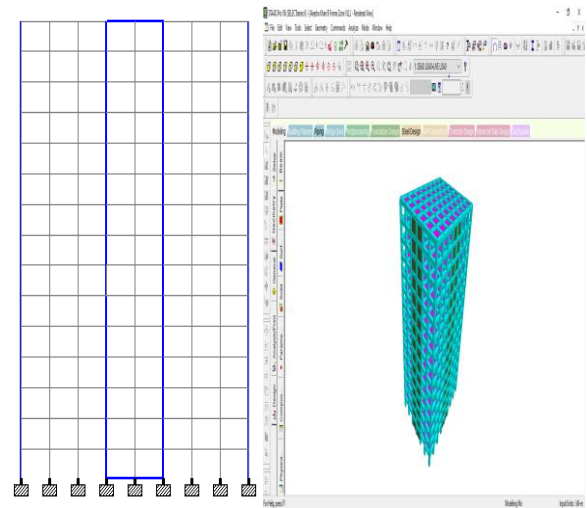


Figure 9. Framed building with Shear wall at the exterior side along X and Z-direction.

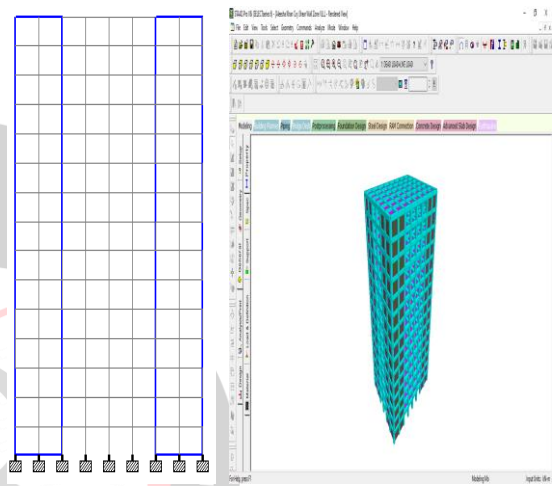


Figure 10. Framed building with Shear wall at the exterior side around the corners

VI. RESULTS

The result is based on the responses of the bare frame model and the changes in the responses after using bracings and shear wall. The results include changes in time periods for axial forces, moments, max shear force and max torsion for along X and Z direction considered individually for different earthquake intensities II, III, IV and V by response spectra method

A. Maximum Axial Force on columns

The maximum axial force for structures are presented in Table-2.

Table-2: Maximum Axial Force for Zone- Zone- II, III, IV & V

Zone	Soil Type	Model Type	Max. Moment KN-m in Z- dir
II	Medium	BFB	126.363
		BX1	112.452
		BY2	102.523
		BXY3	116.405

		BEC4	117.432
		SW1	119.843
		SW2	124.632
		SW3	125.873
		SW4	82.213
III	Medium	BFB	178.965
		BX1	146.952
		BY2	154.864
		BXY3	159.845
		BEC4	163.985
		SW1	170.853
		SW2	187.453
		SW3	103.856
		SW4	147.976
		IV	Medium
BX1	225.941		
BY2	208.879		
BXY3	113.834		
BEC4	226.832		
SW1	239.987		
SW2	243.767		
SW3	227.654		
SW4	137.856		
V	Medium		
		BX1	475.987
		BY2	330.989
		BXY3	311.432
		BEC4	313.856
		SW1	328.675
		SW2	347.342
		SW3	351.543
		SW4	427.879

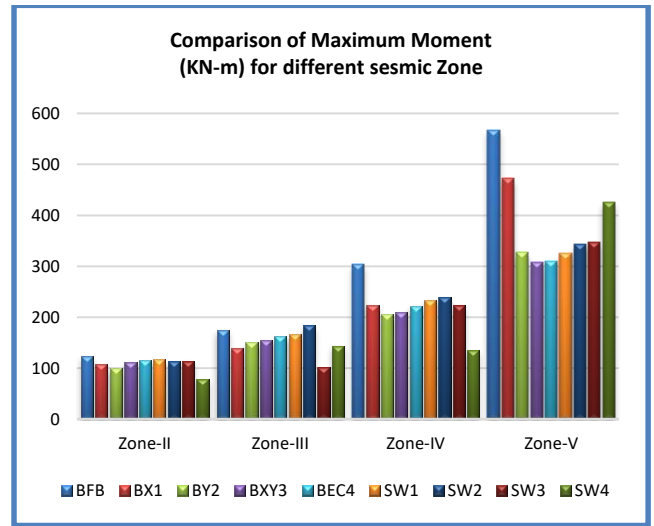


Figure 5.12: Comparison of Maximum Moment in columns

Table-3. Comparison of Maximum Moment in columns for Zone- Zone- II, III, IV & V

C. Maximum Torsion in beams

The maximum Torsion for structures are presented in Figure 13

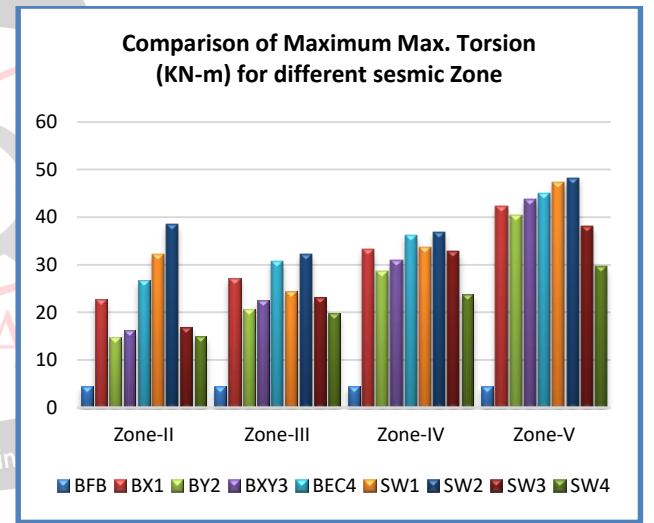


Figure 13: Comparison of Maximum Torsion

D. Maximum Shear Force in beams

The maximum shear force for structures are presented in Figure 14

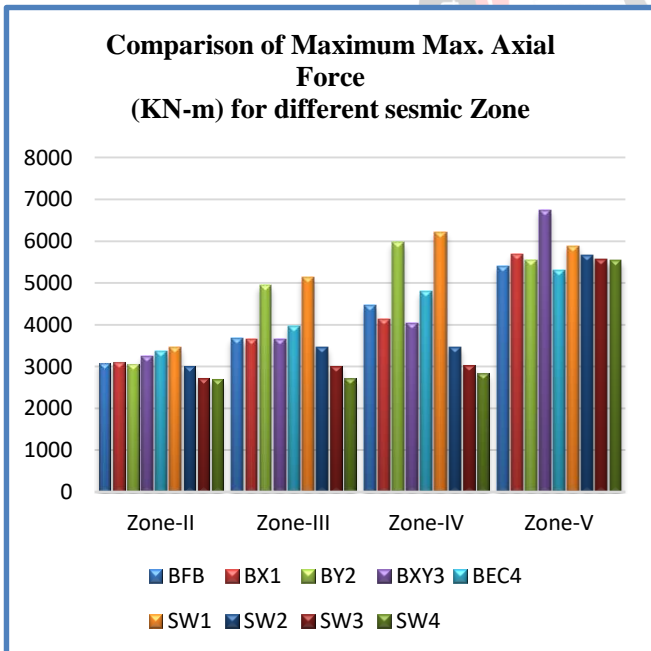


Figure 11: Comparison of Maximum Max. Axial Force

B. Maximum Moment in columns

The maximum moment for structures are presented in Table-3

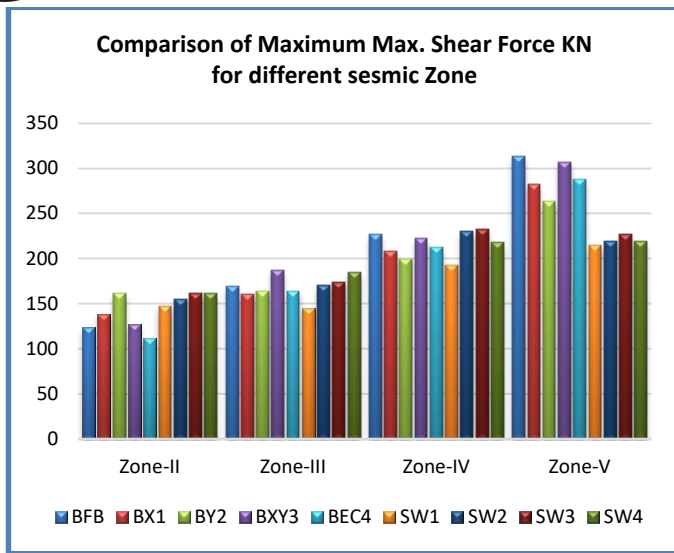


Figure 14: Comparison of Maximum Max. Shear Force

E. Maximum Bending Moments in Beam

The maximum Bending moments for structures are presented in Figure 15

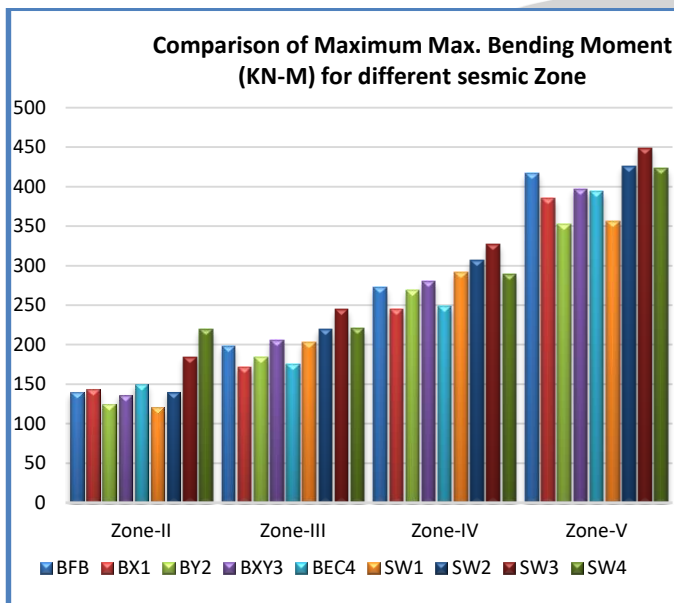


Figure 15: Comparison of Maximum. Bending Moment

VII. CONCLUSIONS

In this study, the analysis of multistoried buildings are done by STAAD PRO software using response spectrum analysis and we have got the following conclusions.

1. The space of shear-divider and backing part immensely affects the seismic response than the plane packaging.
2. Shear divider advancement will give huge strength to the construction by diminishing the mischief to the plan.
3. Shear divider parts are a ton of capable in lessening sidelong migration of edge as buoy and level redirection impelled in shear divider diagram are

altogether not exactly that incited in upheld edge and plane edge.

4. The space of shear-divider (SW4) is ideal as they are feasible in diminishing exercises incited in diagram with less even redirection and buoy.
5. Shear divider improvement will give gigantic immovability to the design by diminishing the mischief to the development.
6. The thought of using steel supporting is one of the positive thoughts which can be used to strengthen or retrofit the current plans.
7. Steel bracings can be used as a choice as opposed to the following building up or retrofitting techniques available as the hard and fast weight on the current construction will not change basically.
8. Steel bracings reduce flexure and shear demands on shafts and areas and move the sidelong loads through center point load framework.
9. The sidelong movements of the design pondered are reduced by the usage of X sort of supporting structures.
10. The construction diagrams with X supporting structure will have least possible bowing minutes interestingly with various kinds of setting systems.
11. Using steel bracings the total burden on the current design will not change on a very basic level.
12. The equal evacuating of the design is diminished by 35% to 45 % by the use of X Type steel supporting structure, and X setting type reduced most outrageous expulsion.

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